

Buttress Dams

Best Practices in Dam and Levee Safety Risk Analysis

Part E – Concrete Structures

Chapter E-5

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US Army Corps
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Outline

- Load Carrying Mechanism and Types
- Case Histories
- Seismic Considerations
- Typical Event Tree (Seismic)
- Reinforcing Steel Considerations
- Hydrodynamic Interaction
- Evaluating Reinforced Concrete Members
- Nonlinear Analysis



Objectives

- Understand the mechanisms that affect buttress dam failure
- Understand how to construct an event tree to represent buttress dam failure
- Understand how to estimate nodal probabilities and probability of breach



Key Concepts

- Buttress dams constructed mainly in early 20th century when labor was cheap and materials were expensive.
- Buttresses saved on concrete but light structures required upstream sloping water barriers – water force acting downward needed for stability.
- Designed to carry load in stream direction, but did not consider (seismic) loading in cross-stream direction.
- Cracking or yielding of reinforced concrete members does not equal dam failure

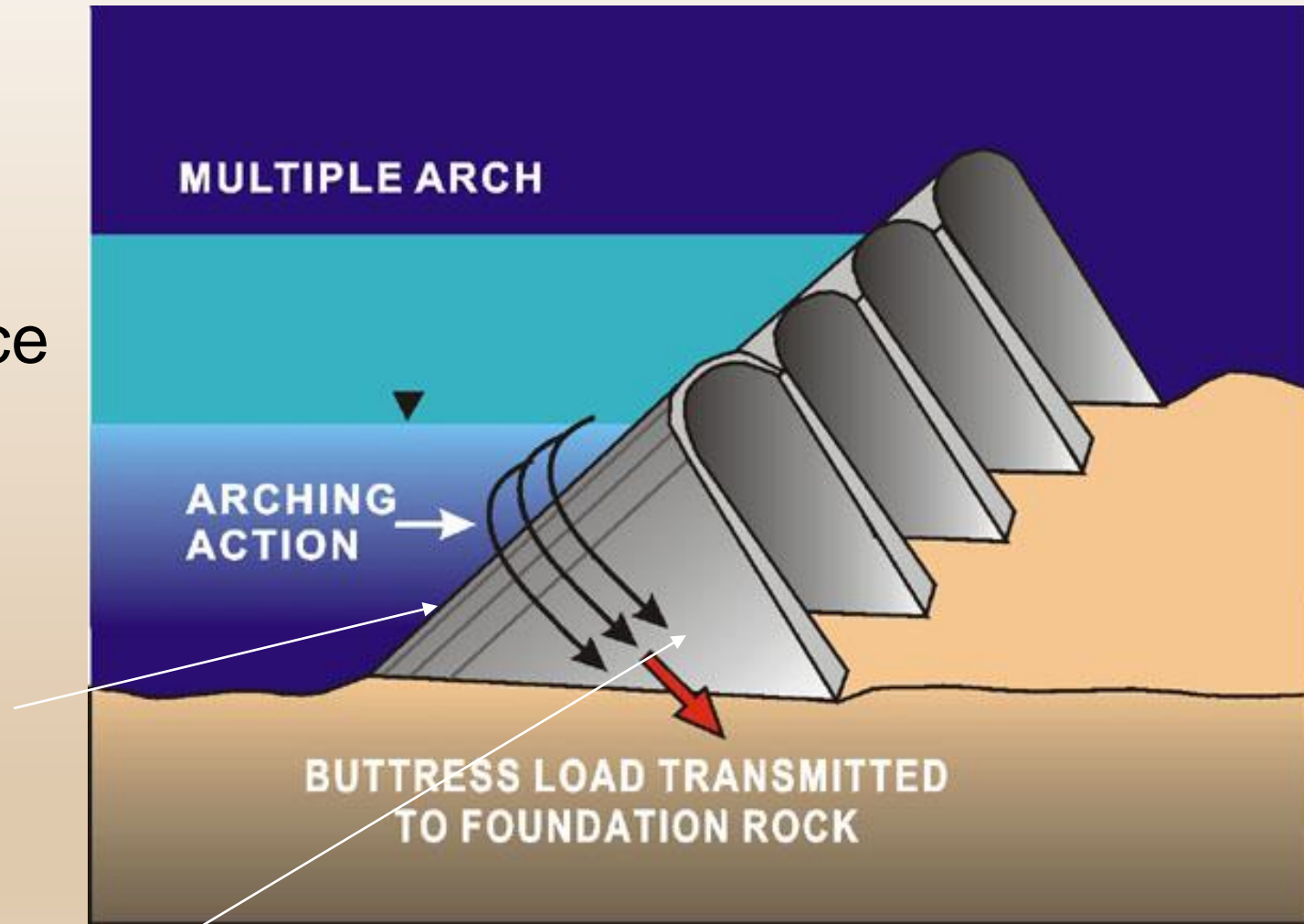


Load Carrying Mechanism and Types



Buttress Dams

Early 20th century
Expensive materials
Cheap labor
Sloped upstream face
needed for stability



Buttresses

Slab and Buttress



Also known as an Amburesen Dam

Multiple Arch



Massive-Head Buttress



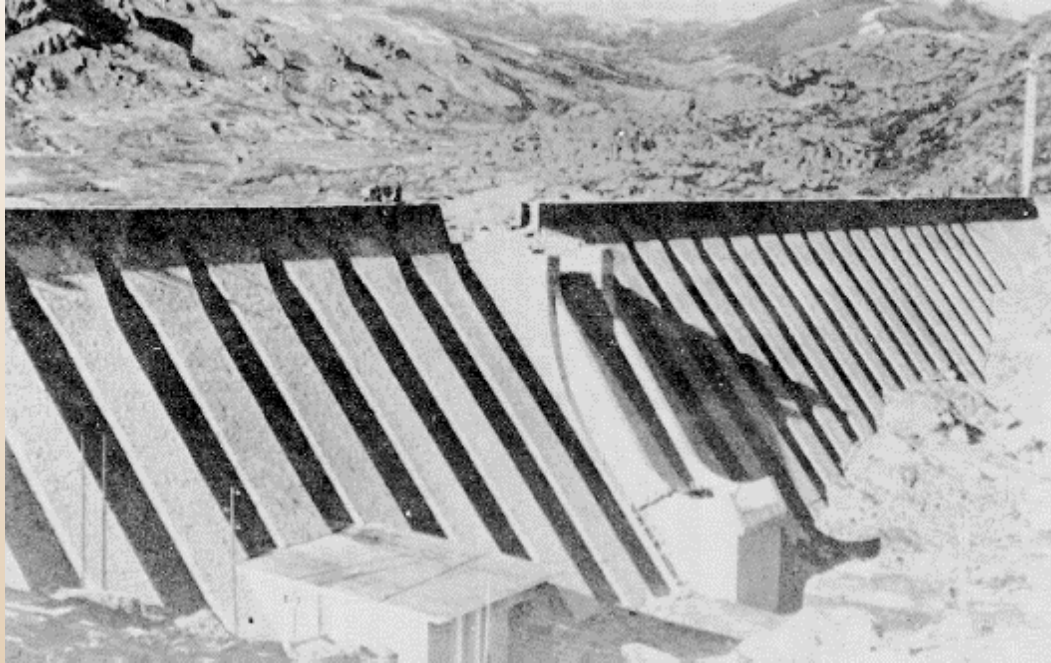
Domes



Case Histories



Vega de Tera Dam, Northwest Spain



112' high buttress dam
completed 1957

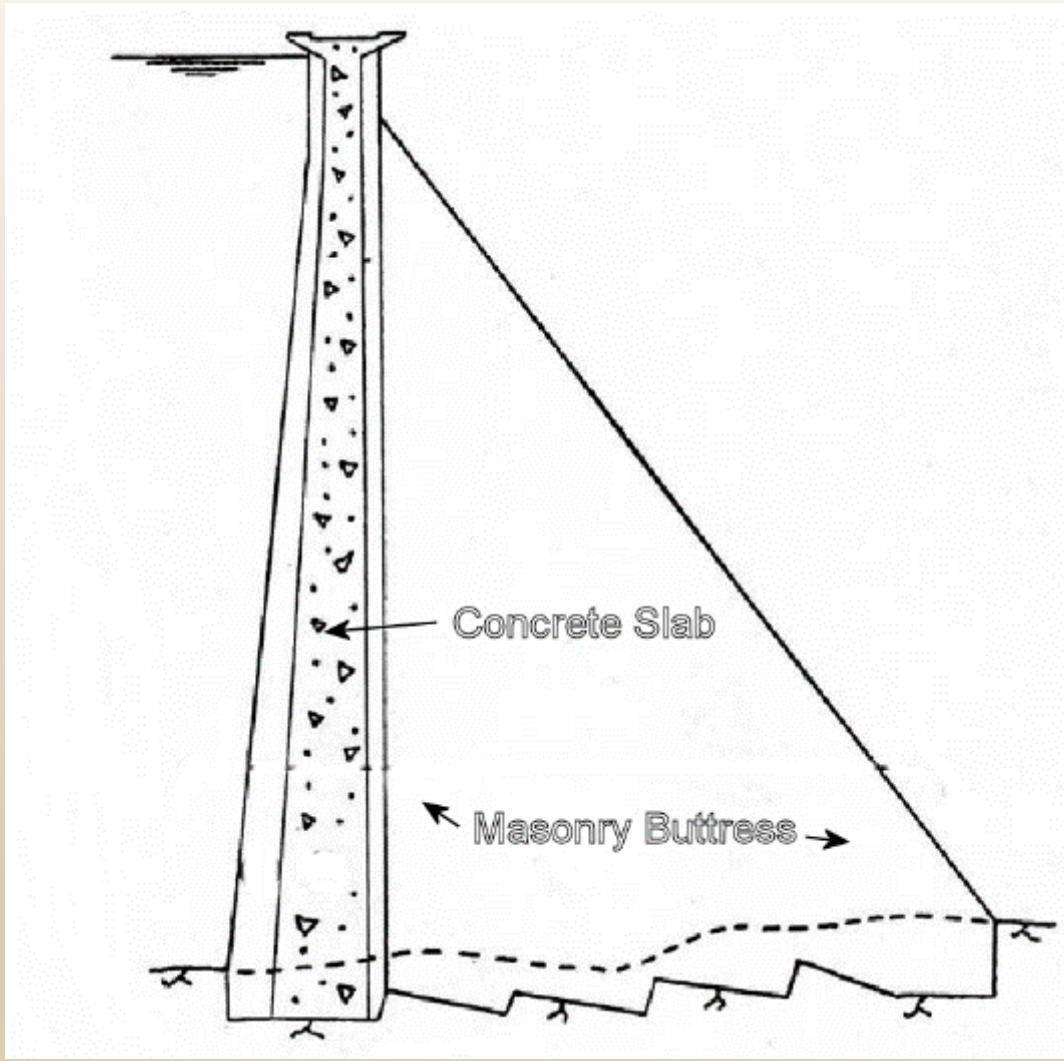
Winter shutdown, little
attention to lift joints

Failed January 10, 1959

144 fatalities



Vega de Tera Dam



But not much slope on u/s face!

Buttresses cement mortared masonry

Grouting in 1956 to control leakage

Reservoir full in 1958

Empty in October 1958

In January, heavy rains filled reservoir

17 buttresses failed in rapid succession

Failure initiated between masonry and concrete on sloping portion of foundation

Modulus of masonry = 140,000 lb/in²

Official cause of failure attributed to large deformations due to low modulus masonry

Gleno Dam, Italy



- 164-ft high multiple arch
- 52-foot high masonry plug constructed in deep central gorge (lime mortar instead of specified cement mortar)
- Original design called for gravity dam; design changed and dam built prior to approval
- Poor concrete quality
- Dam survived nearly full for 2 months
- Failed October 22, 1923
- 100-foot high wave, widespread destruction in Dezzo River Valley
- 356 fatalities

Poor Concrete Quality



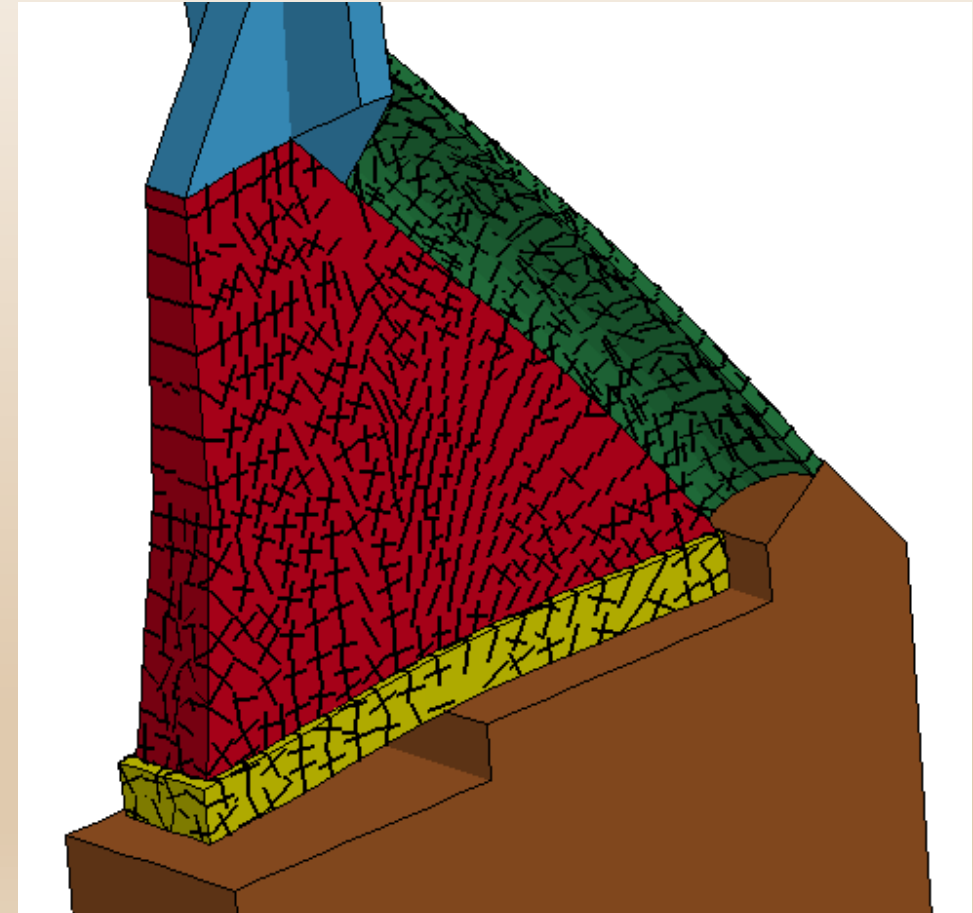
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Gleno Dam Failure – Nonlinear FE Model

- Official inquiry indicated the masonry plug was not stiff enough nor did it extend far enough downstream to carry the buttress loads.
- With complete loss of contact over the front 35 ft of foundation (representing the downstream portion of the masonry plug), the finite model depicted here fails catastrophically!



Gleno Dam



Masonry
Plug

Model predicted cracking
matched observed cracking



Seismic Considerations



Typical Event Tree for Seismic Evaluation

- ↳ Reservoir at or above threshold level
- ↳ Cross stream earthquake load range
 - ↳ Struts fail in compression
 - ↳ Buttress moment capacity exceeded
(concrete cracks/reinforcement fails)
 - ↳ Buttress buckles or deforms excessively
(upstream water barrier lost)
 - ↳ Breach outflow loads adjacent
buttresses, multiple buttresses fail

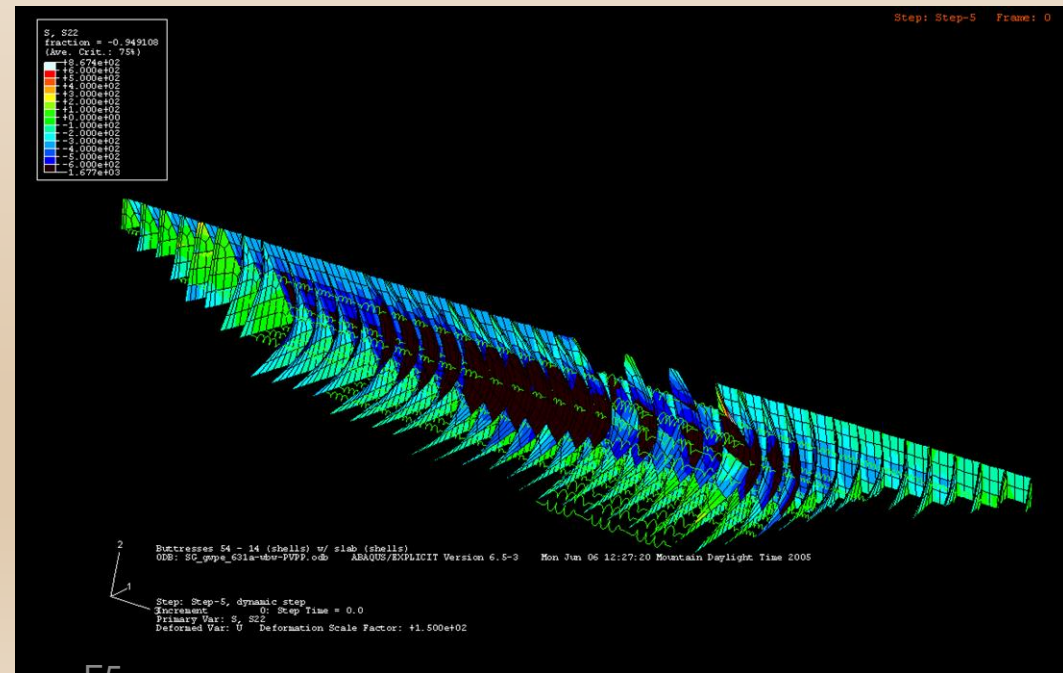
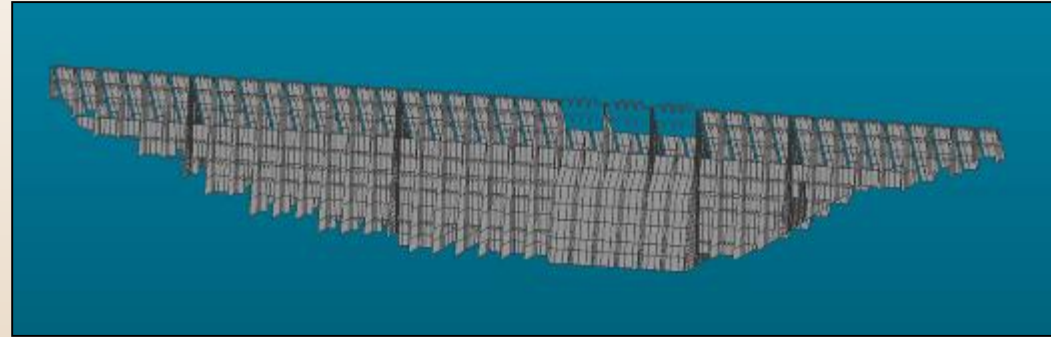
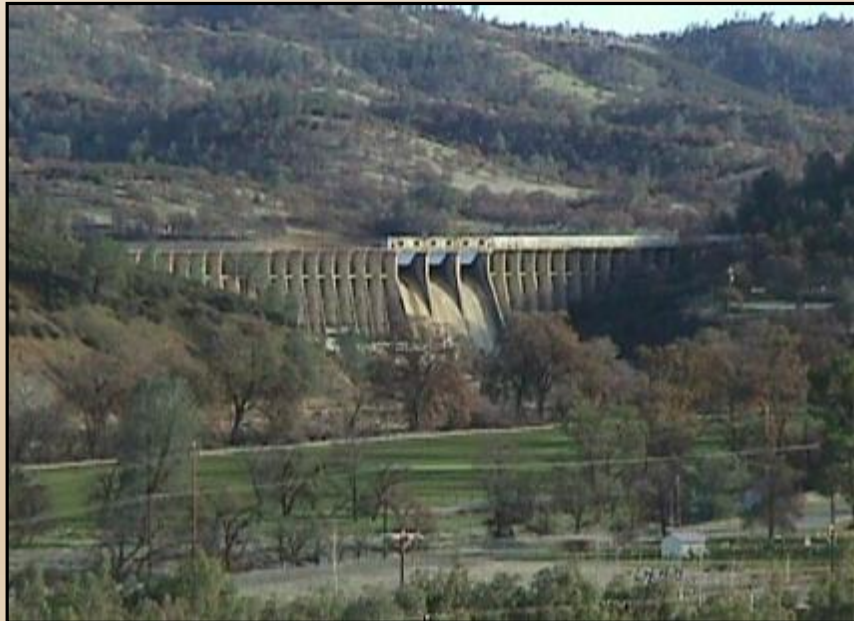
Water Barrier Evaluation

- Note that the previous event tree did not address the upstream slabs/corbels, arches, or domes
- This would be a separate potential failure mode that should be evaluated using reinforced concrete principles (covered in another chapter)



Buttress Dam Seismic Analysis

- Buttresses weak in cross-canyon direction
- Entire dam must be modeled to include “racking”



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Analysis Progression

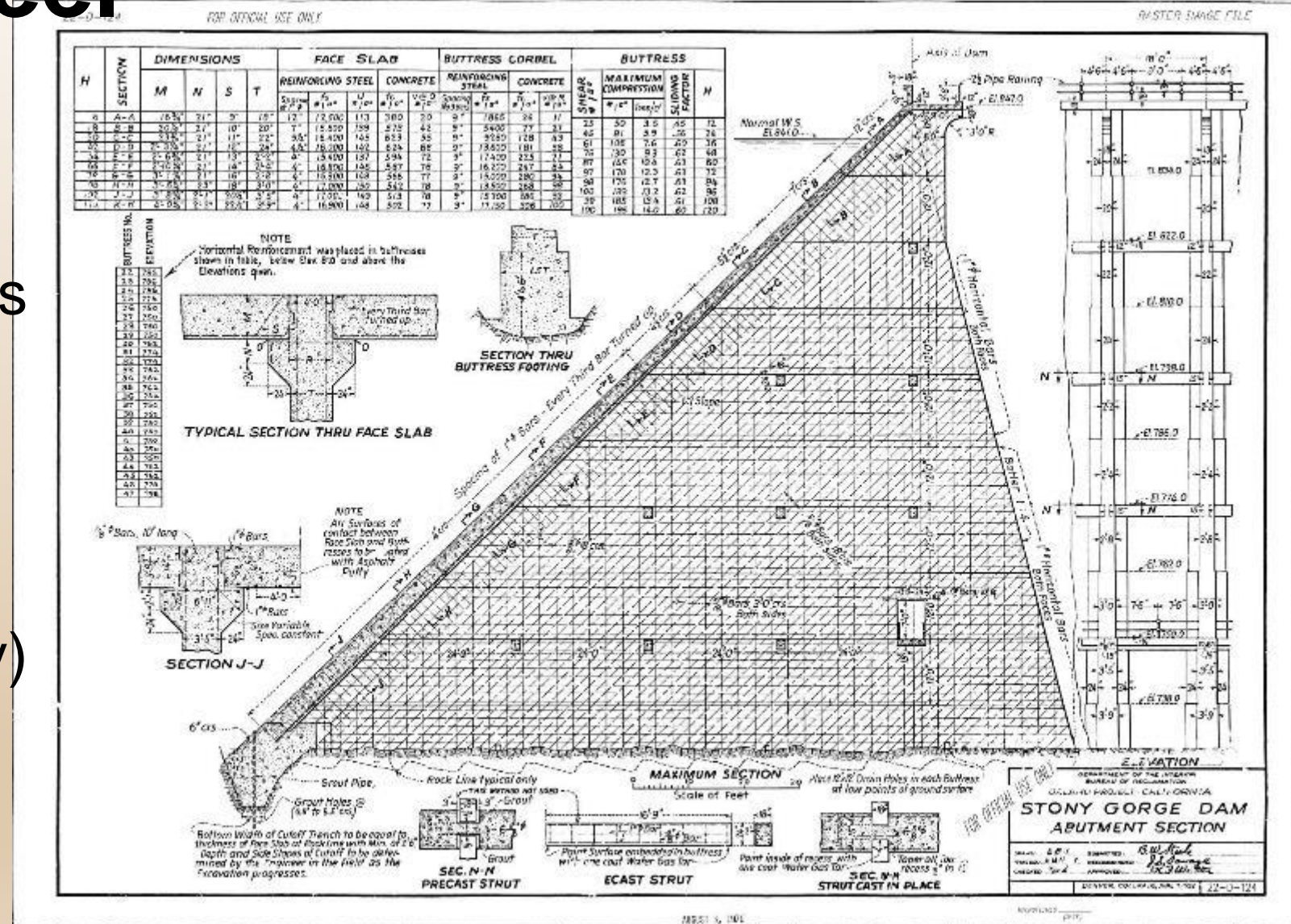
- Note: Although buttress dams typically require a 3-D finite element analysis to draw any meaningful conclusions, initial analyses should be as simple as possible (e.g. linear-elastic, massless foundation and added mass for hydrodynamic interaction) and progress to more sophisticated analyses as needed.



Reinforcing Steel

Most buttress dams are getting old, concrete spalls or cracking can lead to corrosion

Also no air entrainment (freeze-thaw susceptibility) and reinforcement not up to current standards



Hydrodynamic Interaction, Stream Direction

The hydrodynamic forces are reduced by the sloping face (the water will tend to ride up along the face). Zangar approach (below) can be used with directional masses.

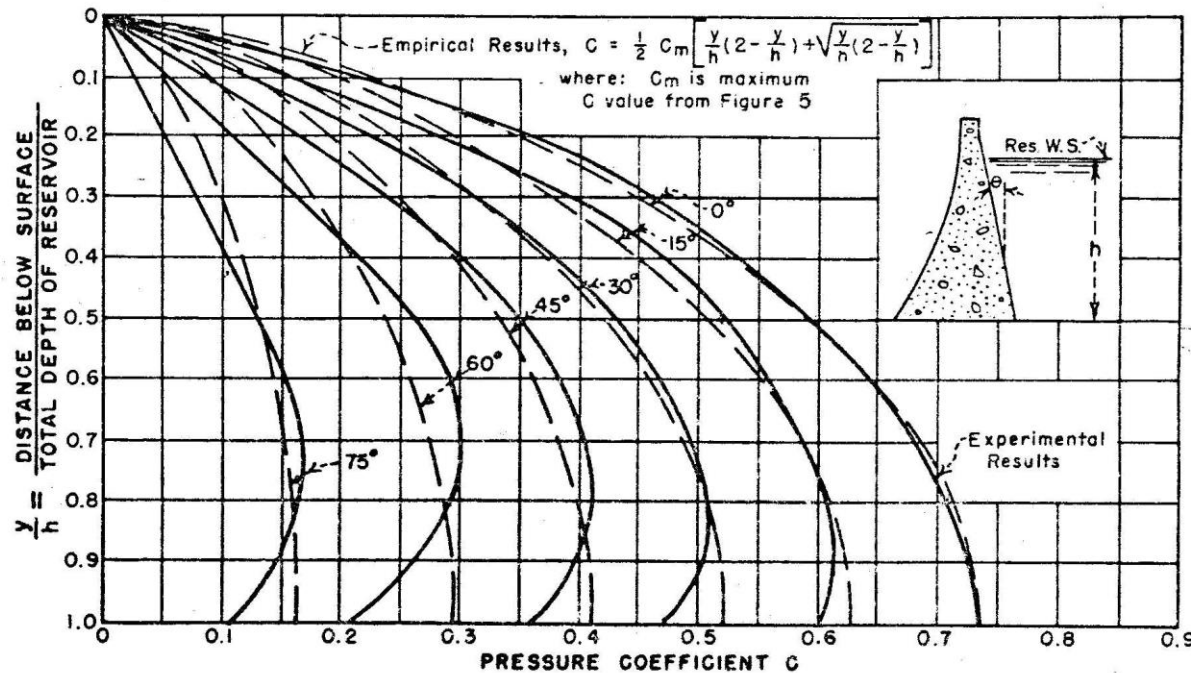


FIGURE 6 - Comparison of experimental and empirical pressure distribution curves.

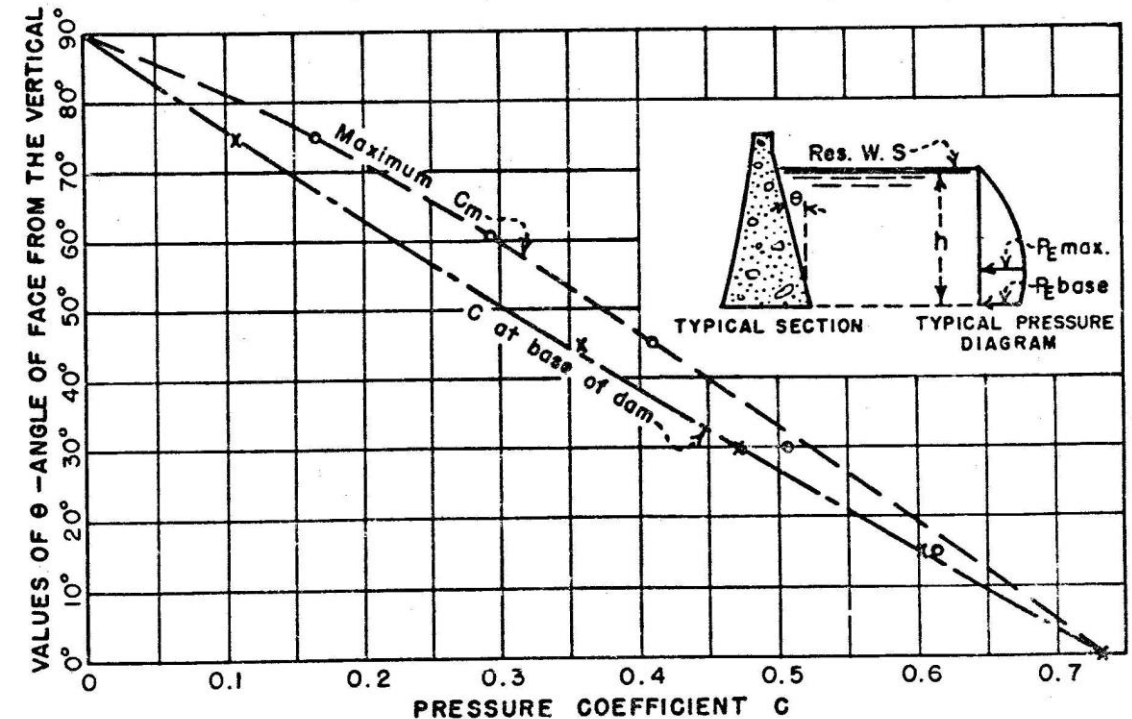


FIGURE 5 - Pressure coefficients for constant sloping faces.

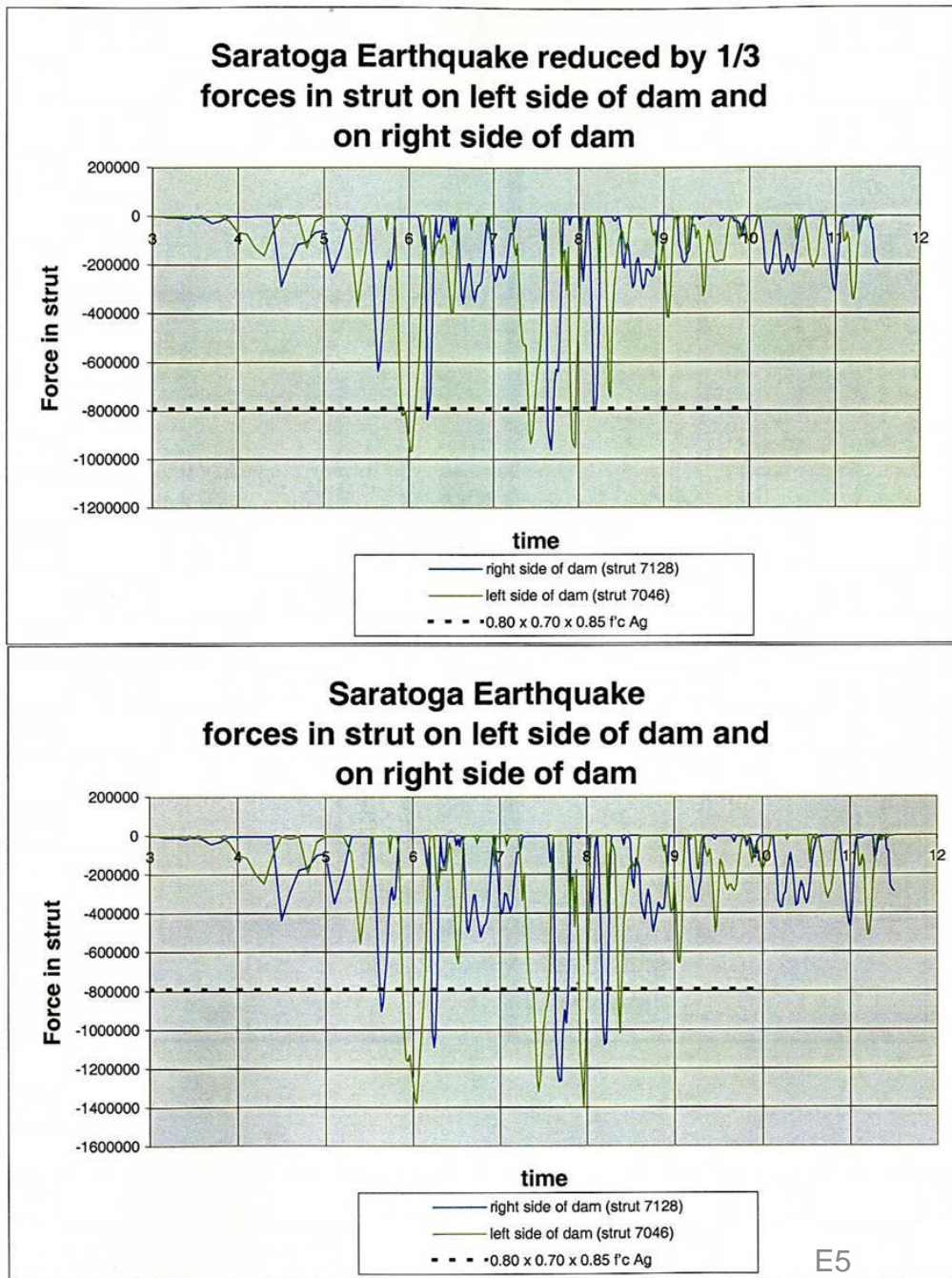
Or use fluid elements, if available

Struts

Some buttresses have struts between buttresses for lateral support

As buttresses move in earthquake, load accumulates across dam and can overload end struts (push over analysis)

Crushing stress in struts normally controls over buckling



Condition of Struts during 10k Motions

Struts for which output was requested:

Buttress	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44	45	46	47	48	49	50	51	52
Elevation 822	7002	7003	7004	7005	7006	7007	7008	7009	7010	7011	7012	7013							7020	7021	7022	7023	7024	7025	7026	7027	7028	7029	7030	7031	7032	7033	7034	7035	7036	7037	7038	7039	7040
Elevation 798 ds						7045	7046	7047	7048	7049	7050	7051	7052	7053	7054	7055	7056	7057	7058	7059	7060	7061	7062	7063	7064	7065	7066	7067	7068	7069	7070	7071	7072	7073	7074	7075	7259	7261	
Elevation 798 us								7076	7077	7078	7079	7080	7081	7082	7083	7084	7085	7086	7087	7088	7089	7090	7091	7092	7093	7094	7095	7096	7097	7098	7099	7100	7101	7102	7103	7104	7260		
Elevation 774 ds								7253	7105	7106	7107	7108	7109	7110	7111	7112	7113	7114	7115	7116	7117	7118	7119	7120	7121	7122	7123	7124	7125	7126	7127	7128	7256						
Elevation 774								7254	7129	7130	7131	7132	7133	7134	7135	7136	7137	7138	7139	7140	7141	7142	7143	7144	7145	7146	7147	7148	7149	7150	7151	7152	7257						
Elevation 774 us								7255	7153	7154	7155	7156	7157	7158	7159	7160	7161	7162	7163	7164	7165	7166	7167	7168	7169	7170	7171	7172	7173	7174	7175	7176	7258						
Elevation 750 ds																																							
Elevation 750																																							
Elevation 750																																							
Elevation 750 us																																							

Struts selected for time history output

First struts removed (Saratoga EO)

(due to exceedance of capacity at first pulse)

Buttress	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44	45	46	47	48	49	50	51	52
Elevation 822	7002	7003	7004	7005	7006	7007	7008	7009	7010	7011	7012	7013							7020	7021	7022	7023	7024	7025	7026	7027	7028	7029	7030	7031	7032	7033	7034	7035	7036	7037	7038	7039	7040
Elevation 798 ds						7045	7046	7047	7048	7049	7050	7051	7052	7053	7054	7055	7056	7057	7058	7059	7060	7061	7062	7063	7064	7065	7066	7067	7068	7069	7070	7071	7072	7073	7074	7075	7259	7261	
Elevation 798 us								7076	7077	7078	7079	7080	7081	7082	7083	7084	7085	7086	7087	7088	7089	7090	7091	7092	7093	7094	7095	7096	7097	7098	7099	7100	7101	7102	7103	7104	7260		
Elevation 774 ds						7253	7105	7110	7111	7112	7113	7114	7115	7116	7117	7118	7119	7120	7121	7122	7123	7124	7125	7126	7127	7128	7129	7130	7131	7132	7133	7134	7135	7136	7137	7138	7139	7140	
Elevation 774						7254	7129	7130	7131	7132	7133	7134	7135	7136	7137	7138	7139	7140	7141	7142	7143	7144	7145	7146	7147	7148	7149	7150	7151	7152	7257								
Elevation 774 us						7255	7153	7154	7155	7156	7157	7158	7159	7160	7161	7162	7163	7164	7165	7166	7167	7168	7169	7170	7171	7172	7173	7174	7175	7176	7258								
Elevation 750 ds						7243	7177	7178	7179	7180	7181	7182	7183	7184	7185	7186	7187	7188	7189	7190	7191	7192	7193	7194															
Elevation 750						7250	7195	7196	7197	7198	7199	7200	7201	7202	7203	7204	7205	7206	7207	7208	7209	7210	7211	7212															
Elevation 750						7252	7213	7214	7215	7216	7217	7218	7219	7220	7221	7222	7223	7224	7225	7226	7227	7228	7229	7230															
Elevation 750 us						7251	7231	7232	7233	7234	7235	7236	7237	7238	7239	7240	7241	7242	7243	7244	7245	7246	7247	7248															

Struts removed due to exceedance of capacity at first pulse of Saratoga Record

From analysis with above struts removed,
the following additional struts reached loads exceeding capacity

Buttress	14	15	16	17	18	19	20	21	22	23	24	25	26	27	28	29	30	31	32	33	34	35	36	37	38	39	40	41	42	43	44	45	46	47	48	49	50	51	52
Elevation 822	7002	7003	7004	7005	7006	7007	7008	7009	7010	7011	7012	7013							7020	7021	7022	7023	7024	7025	7026	7027	7028	7029	7030	7031	7032	7033	7034	7035	7036	7037	7038	7039	7040
Elevation 798 ds						7045	7046	7047	7048	7049	7050	7051	7052	7053	7054	7055	7056	7057	7058	7059	7060	7061	7062	7063	7064	7065	7066	7067	7068	7069	7070	7071	7072	7073	7074	7075	7259	7261	
Elevation 798 us								7076	7077	7078	7079	7080	7081	7082	7083	7084	7085	7086	7087	7088	7089	7090	7091	7092	7093	7094	7095	7096	7097	7098	7099	7100	7101	7102	7103	7104	7260		
Elevation 774 ds								7253	7105	7106	7107	7108	7109	7110	7111	7112	7113	7114	7115	7116	7117	7118	7119	7120	7121	7122	7123	7124	7125	7126	7127	7128	7256						
Elevation 774								7254	7129	7130	7131	7132	7133	7134	7135	7136	7137	7138	7139	7140	7141	7142	7143	7144	7145	7146	7147	7148	7149	7150	7151	7152	7257						
Elevation 774 us								7255	7153	7154	7155	7156	7157	7158	7159	7160	7161	7162	7163	7164	7165	7166	7167	7168	7169	7170	7171	7172	7173	7174	7175	7176	7258						
Elevation 750 ds								7249	7177	7178	7179	7180	7181	7182	7183	7184	7185	7186	7187	7188	7189	7190	7191	7192	7193	7194													
Elevation 750								7250	7195	7196	7197	7198	7199	7200	7201	7202	7203	7204	7205	7206	7207	7208	7209	7210	7211	7212													
Elevation 750								7251	7213	7214	7215	7216	7217	7218	7219	7220	7221	7222	7223	7224	7225	7226	7227	7228	7229	7230													
Elevation 750 us								7252	7239	7240	7241	7242	7243	7244	7245	7246	7247	7248																					

Struts removed due to exceedance of capacity at first pulse (red) of Saratoga Record

Additional struts exceeding capacity (blue)

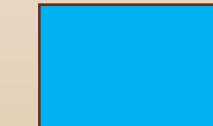
Struts for which forces were reduced (purple)



Output
Selected



1st
failures
removed

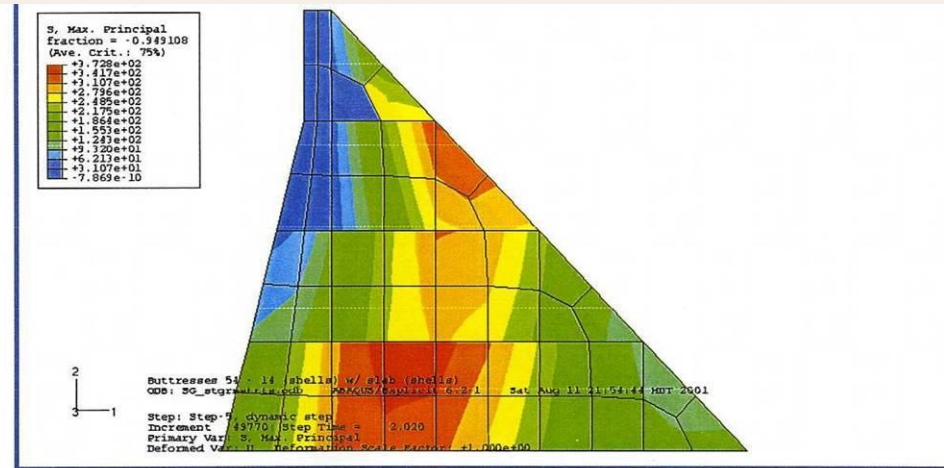


2nd
failures
removed

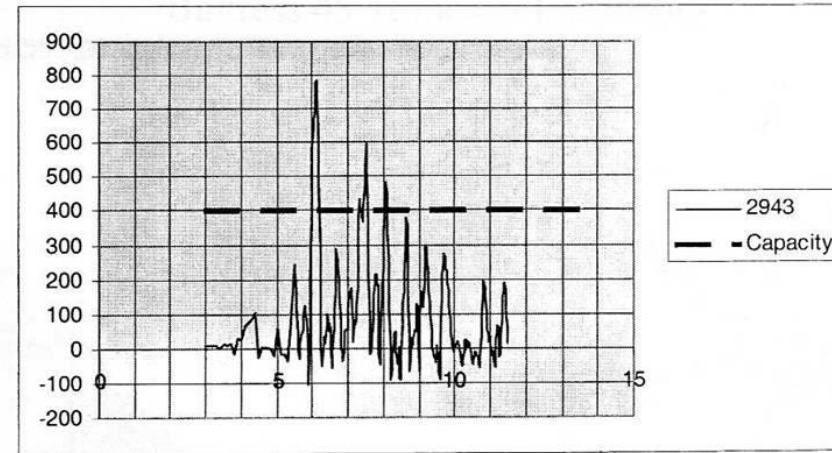
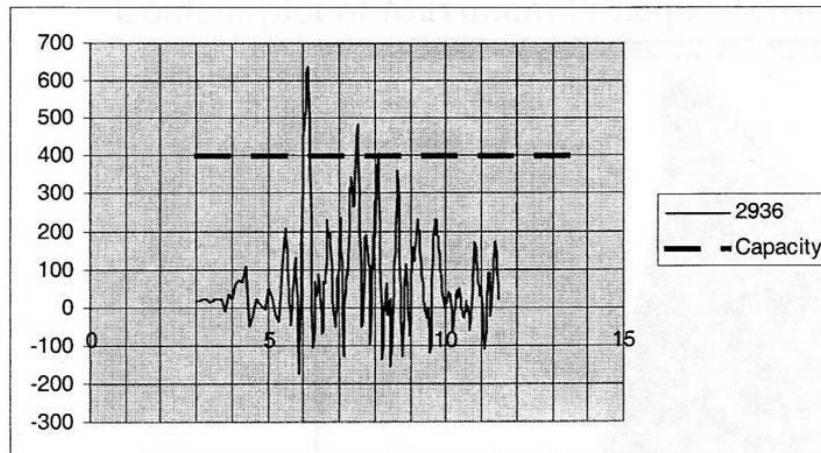


2nd near
failures
removed

Condition of Buttresses



Contour plot of Maximum Principal Tensile Stresses following Strut Removal – Buttress 45 (time= 7.52 sec)
Saratoga Ground Motions



Time history plot of Maximum Principal Tensile Stresses following Strut Removal – Buttress 25

Moments

As a result of buttress dams varying in thickness and reinforcement from base to crest, the response of the dam and moments in the buttresses will vary.

Seismically Induced Moments in Buttress

ABAQUS

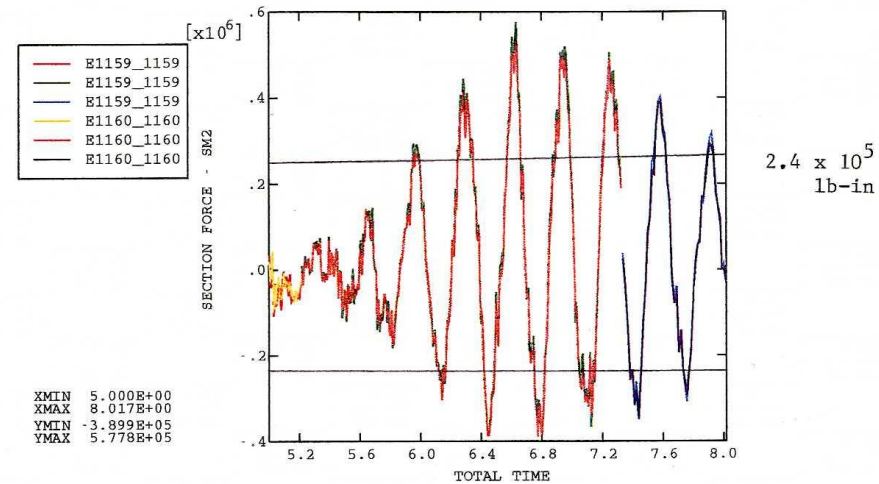


Figure 25a-1 Buttress thickness = 32 inches

Moment History
Figures

ABAQUS

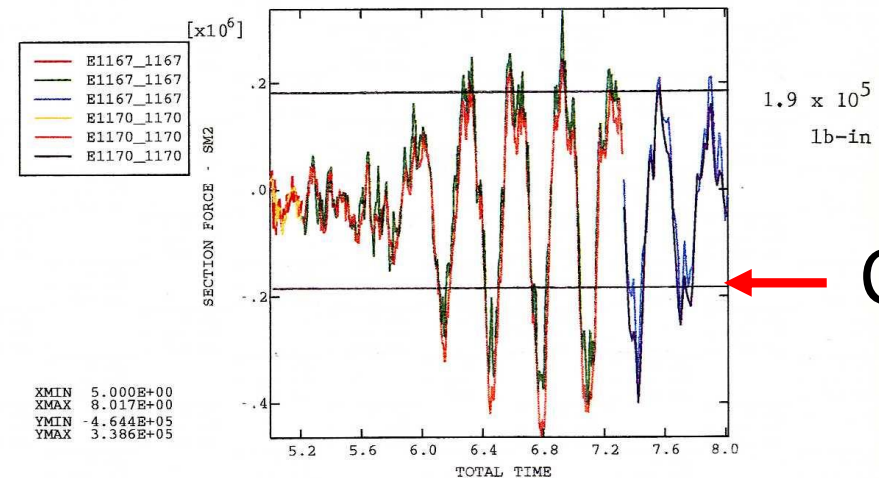
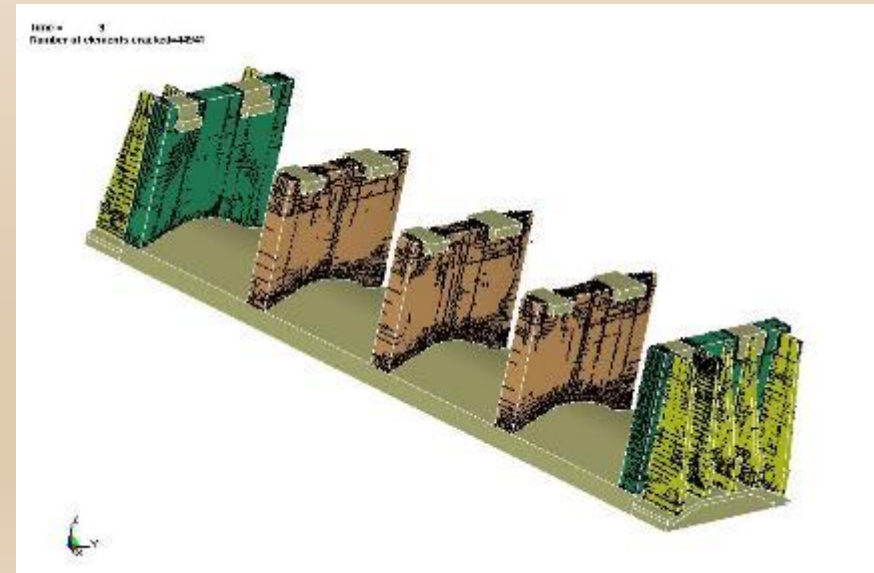


Figure 25a-2 Buttress thickness = 26 inches

Nonlinear Structural Analysis

- Some nonlinear finite element programs have concrete cracking and steel reinforcement models that can be used to examine the potential for cracking, yielding, excessive deformation and failure directly. However, remember these are just models and careful scrutiny of the input assumptions and output is necessary.



Takeaway Points

- Buttress dams designed to carry load in the stream direction
- Buttress dams are vulnerable to cross-stream seismic loads
- Finite element analyses of entire structure are needed to capture response
- Reinforced concrete risk concepts can be used to examine probability of nodal estimates
- Careful consideration of the concrete quality, joint treatment, and reinforcing details are required
- Level of analysis for estimating performance of the buttress dam needs to be commensurate with the level of study needed for the risk analysis and decision-making



Questions or Comments?



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